

ANALYSIS	TRIAL NO. 1		PRE-TENSIONED				
0	GROSS SECTION PROPERTIES - GIRDER		PROPERTIES CALCULATED FROM BUILT-UP SECTIONS				
	AREA	YS	(ITOT = I00 + AY2 - SUM (AY * YB))				
	(10 ³ mm ²)	(mm)	AY	AY2	I00	ITOT	
	(10 ³ mm ³)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	
	GROSS GDR SEC	441	541	238609	140434	38896	50300
0	GROSS COMPOSITE SECTION PROPERTIES		PROPERTIES CALCULATED FROM BUILT-UP SECTIONS				
	AREA	YS	(ITOT = I00 + AY2 - SUM (AY * YB))				
	AY	AY2	I00	ITOT			
	GIRDER ONLY	441	541	238609	140434	38896	50300
	SLAB ONLY	460	1200	552000	562400	1533	
	FILLET ONLY	0	0	0	0	0	
	COVERG ONLY	0	0	0	0	0	
	COMP SEC PROP	901	877	790609	802834	40430	149712
CALCULATED SIMPLY SUPPORTED BEAM MOMENTS							
	X01 = DEAD LOAD OF GIRDER ONLY	=	546.4				
	X02 = DEAD LOAD OF SLAB + FILLET	=	572.5				
	X03 = ADDED DEAD LOAD	=	290.3	INPUTTED VALUE			
	X04 = LIVE LOAD MOMENT	=	1103.1	HS20-44 TRUCK LOADING			
	X05A = ULTIMATE MOMENT APPLIED	=	4228.4	1.3*(X01+X02+X03) + 1.3*(PS TRUCK)			
PRESTRESSED GIRDER ANALYSIS							
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STRESSES W/O PRESTRESS - USING GROSS CROSS-SECTIONS							
	MOMENT (KN-M)	FT (KPa)	FB (KPa)				
DL GIRDER	546.	6075.	-5874.				
DL SLAB + FILLET	572.	6364.	-6154.				
ADDED DL	290.	444.	-1748.				
TOTAL DL	1417.	12883.	-13775.				
(LL+I)K	1103.	1642.	-6463.				
TOTAL DL + (LL+I)	---	14524.	-20238.				
DESIGN STRESS - F DES (KPa) (GREATER OF THE FOLLOWING)							
	FB - 0.5*SQRT(F'c) * ALL. TEN. FACTOR	=	17601.		FB (DL + ADD. DL)	= 13775.	
NOTE :- REAL CALCULATIONS ARE IN ENGLISH UNITS							
0	FINAL TRANSFORMED GIRDER PROPERTIES		PROPERTIES CALCULATED FROM BUILT-UP SECTIONS				
	AREA	YS	(ITOT = I00 + AY2 - SUM (AY * YB))				
	(10 ³ mm ²)	(mm)	AY	AY2	I00	ITOT	
	(10 ³ mm ³)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	
	GROSS GDR SEC	441	541	238609	140434	38896	50300
	(N-1)*AS	13	100	1303	130	---	
	TRAN GDR SEC	454	528	239912	140564	38896	52758
0	FINAL TRANSFORMED COMPOSITE GIRDER PROPERTIES		PROPERTIES CALCULATED FROM BUILT-UP SECTIONS				
	AREA	YS	(ITOT = I00 + AY2 - SUM (AY * YB))				
	AY	AY2	I00	ITOT			
	TRAN COMP GDR	914	866	791912	802964	40430	157469
	(E*IT)/I**2 = 2.1081			(E*YB)/I**2 = 1.8468			

STRESS W/O PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTION			
	MOMENT (KIP-FT)	FT (KSI)	FB (KSI)
DL GIRDER	545.	5302.	-5469.
DL SLAB + FILLER	572.	6205.	-8730.
APPLIED DL	298.	443.	-1641.
TOTAL DL	1417.	12570.	-12840.
(DL+I)I	1108.	1630.	-6067.
TOTAL DL + (DL+I)	—	14208.	-18907.

DESIGN STRESS - F (KSI) (LARGER OF THE FOLLOWING)

FB - $0.5 \sqrt{f'_c}$ (F/C) * ALL. TEN. STRESS = 16200. FB (DL + APPL. DL) = 12840.

NOTE :- FINAL CALCULATIONS ARE IN ENGLISH UNITS

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PRESTRESSED GIRDER ANALYSIS

PRESTRESSING FORCE - PRESTRESSING

ALLOW. TENSION = $(0.75 * .75) - 241.50 = 1154913.09$ FINAL PRESTRESS FORCE - $P_f = 250722.00$

AREA OF PRESTRESSING STEEL - $A_{ps} = 2.17 (10^3 \text{ in}^2)$ INITIAL PRESTRESS FORCE - $P_i = 283628.00$

STRESS WITH PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTION (KSI)

AT MIDSPAN	TOP FIBER	BOTTOM FIBER
§ STRESSING (FTI)	-997.	(FTI) 12592.
§ DL SUPER-SLAB (FTF)	6011.	(FTF) 5066.
§ FULL DES. LOAD (FTF)	6092.	(FTF) -2642.

AT 1/3 POINT ALONG SPAN LENGTH STRESS RUPPED AT 1/3 POINTS

(ASSUM. DL ON DL STRESS AT 1/3 FT. = $0.80 * \text{STRESS @ CL}$)

§ STRESSING (FTI)	-1649.	(FTI) 13534.
§ FULL DES. LOAD (FTF)	4677.	(FTF) 6206.
§ FULL DES. LOAD (FTF)	6279.	(FTF) -562.

CONCRETE STRESSING REQUIRED (KSI)

$P_{ci} = 12840 / .60 = 21400.$ $P_i = 283628 / .40 = 709072.$ $P_{max} (KSI) = 27994.$

DEFLECTIONS (mm) (DOWNWARD POSITIVE)

DL = -41.60 DL+I = 18.10 DL+I+P = -23.50 DL+I+P+DL = -25.36 DL+I+P+DL+I = 18.96 DL+I+P+DL+I+DL = 3.30 DL+I+P+DL+I+DL+I = -6.11

CHECK ULTIMATE RESISTING MOMENT

MIDN. INCR = .063 ULT. POINT. MOMENT (KIP-FT) = 4289.75

PFD (KSI) = 181066. N.A @ ULTIMATE LOAD (mm) = 104.50

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PRESTRESSED GIRDER ANALYSIS

RESULTS - FINAL PRESTRESSING FORCE CALCULATED

GIRDER NO.	STRESSING		CONCRETE STRESSING		FINAL FORCE	ULTIMATE		DEFLECTIONS				SECTION PROPERTIES				
	INITIAL (KSI)	FINAL (KSI)	INIT. (KSI)	FINAL (KSI)		APPLIED (KSI)	RUPD (KSI)	SP1 (mm)	SP2 (mm)	SP3 (mm)	SP4 (mm)	I	A	Y1	I	Y2
0 1	-996	12532	6092	-1641	2507	4228	4290	-41.6	-23.5	18.0	-6.1	52756	454	528	157469	866

NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD

NOTE: DEFLECTIONS ARE BASED ON INPUT P-S F/C

NOTE: FOR P-S CONCRETE F/C USE 27994. MPa

Output for Example Problem 5:

CALIFORNIA DEPARTMENT OF TRANSPORTATION																				PAGE 1			
PRESTRESSED GIRDER ANALYSIS																							
(VERSION 2.00)																							
SECTION DATA - FROM SUPERSTRUCTURE SECTIONS/SECTIONS BY PAGES																							
SECT. NO.	SIGN	TYPE	V	E	X	Y																	
1	0	+	1	8.00	.67	.00	.00																
1	0	+	1	8.00	.67	1.83	.00																
1	0	+	1	.50	1.17	.67	.00																
1	0	+	1	.50	1.17	.67	7.50																
1	0	+	8	.33	.33	.67	7.50																
1	0	+	7	.33	.33	1.83	7.50																
PRESTRESSED GIRDER ANALYSIS																							
INPUT DATA																							
										P													
										X													
										E													
										T													
										M													
T S											O												
R E											N												
I C																							
A T	SPAC	GIR	GIR	P-C	SPAC	--	COMPOSITE	SLAB	--	P S	C	N	A	ALL	TYPE	LOAD	INITIAL	INT.	FINAL	FINAL			
L N	DEPTH	DEPTH	SPAC	P'C	SPAC	THICK	MOD	DATA	P'C	E N	P'S	I	D	X	DIR	FORCE	LINES	SECTION	SECT.	SECT.			
	(IN.)	(IN.)	(IN)	(FT)	(FT)	(IN)	(IN)	(%)	(FT)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(KIP)		(8-FT)	(8-FT)	(8-FT)	(8-FT)		
1 1	104.0	96.0	96	14.0	125	8.0	96	.0	13.3	0 0	1770	3.0	5.4	Y	2.0	0	.00	0	0	740	0		
NOTE: ASSUMED DATA IS INDICATED BY COLONS.																							
ANALYSIS SERIAL NO. 1 POST-DESIGNED																							
GROSS SECTION PROPERTIES - GIRDER (PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)																							
I _{TOP} = I ₁₀ + A ₁₂ - S ₁₂ (A ₁ * Y ₁₂)																							
AREA Y ₁₂ A ₁ A ₁₂ I ₁₀ I _{TOP}																							
(IN ²) (IN) (IN ²) (IN ⁴) (IN ⁴) (IN ⁴)																							
GROSS GJR SEC 1727.8 48.4 83274. 4409311. 1186066. 1552953.																							
GROSS COMPOSITE SECTION PROPERTIES (PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)																							
I _{TOP} = I ₁₀ + A ₁₂ - S ₁₂ (A ₁ * Y ₁₂)																							
AREA Y ₁₂ A ₁ A ₁₂ I ₁₀ I _{TOP}																							
(IN ²) (IN) (IN ²) (IN ⁴) (IN ⁴) (IN ⁴)																							
GIRDER ONLY 1727.8 48.4 83274. 4409311. 1186066. 1552953.																							
SLAB ONLY 768.0 100.0 76800. 7680000. 4096. 0.																							
FILLER ONLY .0 .0 0. 0. 0. 0.																							
CONCRETE ONLY .0 .0 0. 0. 0. 0.																							
COMP SEC PROP 2495.8 64.2 160274. 12089311. 1190162. 2974364.																							
CALCULATED DEAD WEIGHTED BEAM MEMBRS																							
D ₁₀ = DEAD LOAD OF GIRDER ONLY = 3514.4																							
D ₁₂ = DEAD LOAD OF SLAB + FILLER = 1563.0																							
D ₁₅ = ASSED DEAD LOAD = 740.0 INPUTED VALUE																							
D ₁₆ = LIVE LOAD MEMBR = 1722.0 MDL-44 TRUCK LOADING																							
D ₁₇ = ULTIMATE MEMBR APPLIED = 14211.2 1.3*(D ₁₀ +D ₁₂ +D ₁₅) + 1.3*(D ₁₆ TRUCK)																							

PRESTRESSED GIRDER ANALYSIS

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STRESSES W/O PRESTRESS - USING GROSS CROSS-SECTIONS

	MOMENT (K')	FT (PSI)	FB (PSI)
DL GIRDER	3516.	1294.	-1314.
DL SLAB + FILLET	1563.	575.	-584.
ADDED DL	740.	95.	-152.
TOTAL DL	5819.	1964.	-2090.
(LL+I)H	1722.	221.	-446.
TOTAL DL + (LL+I)	---	2185.	-2537.

DESIGN STRESS - F DES (PSI) (GREATER OF THE FOLLOWING)

$$F_D = 6 * SQRT(F'C) * ALL. TEN. FACTOR = 2157. \quad F_D (DL + ADD. DL) = 2090.$$

PRESTRESSED GIRDER ANALYSIS

PAGE 4

FINAL NET TRANSFORMED GIRDER PROPERTIES (PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)

	AREA	YB	AY	AY2	IXO	IXOY
	(IN2)	(IN)	(IN)	(IN4)	(IN4)	(IN4)
GROSS GER SEC	1727.8	48.4	83574.	4409311.	1186066.	1552953.
P/S DECK	-12.8	3.0	-38.	-115.	---	---
NET TRANS GER	1715.1	48.7	83536.	4409196.	1186066.	1552647.

FINAL TRANSFORMED GIRDER PROPERTIES (PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)

	AREA	YB	AY	AY2	IXO	IXOY
	(IN2)	(IN)	(IN)	(IN4)	(IN4)	(IN4)
GROSS GER SEC	1727.8	48.4	83574.	4409311.	1186066.	1552953.
(P-1) * AS'	38.3	3.0	115.	345.	---	---
TRANS GER SEC	1766.2	47.4	83689.	4409656.	1186066.	1630146.

FINAL TRANSFORMED COMPOSITE GIRDER PROPERTIES (PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)

	AREA	YB	AY	AY2	IXO	IXOY
	(IN2)	(IN)	(IN)	(IN4)	(IN4)	(IN4)
TRANS COMP GER	2534.2	63.3	167489.	12089656.	1190162.	3116031.
			$(E*YI)/R**2 = 2.4287$			$(E*YB)/R**2 = 2.5613$

STRESSES W/O PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS

	MOMENT (K')	FT (PSI)	FB (PSI)
DL GIRDER	3516.	1307.	-1346.
DL SLAB + FILLET	1563.	559.	-545.
ADDED DL	740.	93.	-180.
TOTAL DL	5819.	1960.	-2072.
(LL+I)H	1722.	217.	-420.
TOTAL DL + (LL+I)	---	2176.	-2492.

DESIGN STRESS - F DES (PSI) (GREATER OF THE FOLLOWING)

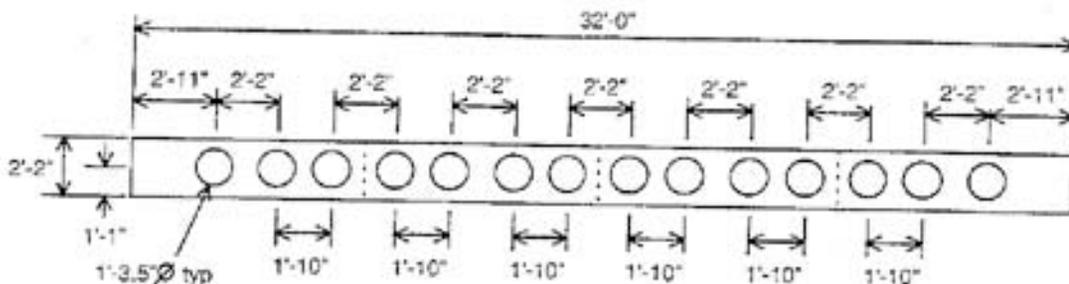
$$F_D = 6 * SQRT(F'C) * ALL. TEN. FACTOR = 2113. \quad F_D (DL + ADD. DL) = 2072.$$

PRESTRESSED GIRDER ANALYSIS										PAGE 5								
PRESTRESSING FORCE - POST-TENSIONED																		
$NIOW, TENSION = (95 * .70) - 20.00 = 16900.00 \text{ (KSI)}$						$NIOW, TENSION = 95 * .60 = 16200.00 \text{ (KSI)}$												
$NIOW, TENSION USED = 16200.00 \text{ (KSI)}$						$FINAL PRESTRESS FORCE - PF = 103501.0 \text{ (LBS)}$												
$AREA OF PRESTRESSING STEEL - ACT = 6.39 \text{ (IN}^2\text{)}$						$INITIAL PRESTRESS FORCE - FI = 120112.5 \text{ (LBS)}$												
STRESS WITH INCREASE - USING FINAL TRANSFORMED CROSS-SECTION (KSI)																		
AT MIDSPAN		TOP FIBER				BOTTOM FIBER												
Ø STRESSING (PSI)	307.	ØFC	1106.															
Ø DL COMPRESSION (PSI)	1005.	ØBF	221.															
Ø FULL DES. LOAD (PSI)	1314.	ØBT	-379.															
CONCRETE STRENGTH REQUIRED (KSI)																		
$FCI - FIMW/.5 = 2009.$			$FCI (ABS) = 3500.$			$FC - 2990/.40 = 3085.$			$FMCW (DES) = 4000.$									
DEFLECTIONS (DOWN) (UPWARD POSITIVE)																		
$IFD = -2.5284$		$ID = 1.6029$		$IF = -.0455$		$IFB = -.0511$		$IDL = .7480$		$IFBL = .2653$								
		$DF = .0023$																
GIRDER ULTIMATE RESISTING MOMENT																		
$RESIST. MOMENT = .023$				$DEF. RESIST. MOMENT (K-FT) = 12066.06$														
$FCI (KSI) = 2000.$				$N.A. Ø ULTIMATE LOAD (20) = 7.44$														
PRESTRESSED GIRDER ANALYSIS										PAGE 6								
RESULTS - FINAL PRESTRESSING FORCE CALLED																		
TOTAL NO.	STRESSING		CONCRETE STRENGTH		FINAL STRESS		DEFLECTION		DEFLECTIONS			SECTION PROPERTIES						
	INITIAL (KSI)	FINAL (KSI)	INT. (KSI)	FINAL (KSI)	AS-BLD (KSI)	POST (KSI)	IFE (FT)	IFB (FT)	IFE (IN)	IFB (IN)	IFE (IN)	IFB (IN)	I (IN ⁴)	A (IN ²)	XB (IN)	YB (IN)		
0 1	307.	1106.	1314.	-379.	2.01	3.29	1035.	14211.2	12886.9	-2.51	-0.70	.022	.007	163045	1766	47.4	312621	61.3
NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD																		
NOTE: DEFLECTIONS ARE BASED ON INPUT P-S P/C																		
NOTE: FOR P-S CONCRETE P/C USE 4000. PSI																		
NOTE: APPLIED ULTIMATE MOMENT EXCEEDS ULTIMATE RESISTING MOMENT. MID-SPAN IS REQUIRED.																		

Note: If parts are divided in a different way, a slightly different result will be achieved. By editing the text file in free formatted mode to the significant digits required to achieve the desired accuracy, the way of analysis will not affect the results.

Example Problem 6 (English)

Cored Slab Section



STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION
CALTRANS DESIGN SYSTEM
PRESTRESSED GIRDER ANALYSIS
 DS-D-0018 (REV. 11/87)

A
English

Truss No.	Section No.	Girder Properties				Composite Slab				Prestress Data				Moments					
		Structure Depth	Girder Depth	Girder Spacing	Span Length	Thick	Widn	Super	Ta	Ta	XCL	XND	Final Prestress Force	Live Load Lanes	Mo A DLM Applied to Initial Section	Mo B DLM Applied to TFD Initial Section	Mo C DLM Applied to Final Section	Mo D (LL+IM) Applied to Final Section	
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	
1	1	17	2	18	15				7	13	13	1						14	12

STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION
CALTRANS DESIGN SYSTEM
PRESTRESSED GIRDER ANALYSIS - SUPERSTRUCTURE SECTION
 DS-D-0018 (REV. 11/87)

B
English

Section No.	Cross Section Location	Ref. Pt. Coord.		S. S. Data		Rish Data		Interior Girders		Exterior Girders				Overhangs				
		X	Y	Width	Depth	Top Thickness	Bottom Thickness	No. of Girders	Top Thickness	Left		Right		Left		Right		
		(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	Type	Factor	Type	Factor	Type	Factor	Type	Factor	
(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)	(24)	(25)	(26)	
1				132.0	121.6	16.0	16.0	2	8	0	9	0	9	0	9	0	9	0

ANALYSIS	TRIAL NO. 1		FRP-DESIGNED				
0	GROSS SECTION PROPERTIES - GIRDER		(PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)				
	AREA	YB	[$I_{TOT} = I_{G1} + A_{G2} - S_{GM} (A_Y * Y_B)$]				
	(IN ²)	(IN)	A _Y	A _Z	I _{G1}	I _{TOT}	
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)	(IN ⁴)	
	GROSS GCR SEC	7373.2	13.0	95557.	1698572.	59520.	519675.
0	GROSS COMPOSITE SECTION PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)				
	AREA	YB	[$I_{TOT} = I_{G1} + A_{G2} - S_{GM} (A_Y * Y_B)$]				
	A _Z	I _{G1}	I _{TOT}				
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)	(IN ⁴)	
	GIRDER ONLY	7373.2	13.0	95557.	1698572.	59520.	519675.
	SLAB ONLY	.0	.0	0.	0.	0.	0.
	FILLET ONLY	.0	.0	0.	0.	0.	0.
	COVER ONLY	.0	.0	0.	0.	0.	0.
	COMP GCR SEC	7373.2	13.0	95557.	1698572.	59520.	519675.
CALCULATED SIMPLY SUPPORTED BEAM MOMENTS							
	M1 = DEAD LOAD OF GIRDER ONLY	=	3457.3				
	M2 = DEAD LOAD OF SLAB + FILLET	=	.0				
	M3 = ADDED DEAD LOAD	=	450.0	IMPOSED VALUE			
	M4 = LIVE LOAD MOMENT	=	2237.0	IMPOSED VALUE			
	M5 = ULTIMATE MOMENT APPLIED	=	5924.8	1.3*(M3+M4+M2) + 2.17*M4			
PRESTRESSED GIRDER ANALYSIS							
STRESSES W/O PRESTRESS - USING GROSS CROSS-SECTIONS							
	MOMENT (K')	FT (PSI)	FB (PSI)				
	DL GIRDER	3457.	1041.	-1035.			
	DL SLAB + FILLET	0.	0.	0.			
	ADDED DL	450.	135.	-135.			
	TOTAL DL	3907.	1177.	-1169.			
	(LL+I)E	2237.	674.	-669.			
	TOTAL DL + (LL+I)	---	1850.	-1839.			
DESIGN STRESS - F DES (PSI) (GREATER OF THE FOLLOWING)							
	FB = $6 * \sqrt{f'_{c1}} (f'_{c1}) * ALL. TEN. FACTOR$	=	1649.	FB (DL + ADD. DL)	=	1169.	
0	FINAL TRANSFORMED GIRDER PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)				
	AREA	YB	[$I_{TOT} = I_{G1} + A_{G2} - S_{GM} (A_Y * Y_B)$]				
	(IN ²)	(IN)	A _Y	A _Z	I _{G1}	I _{TOT}	
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)	(IN ⁴)	
	GROSS GCR SEC	7373.2	13.0	95557.	1698572.	59520.	519675.
	(H-1)*A _S	153.7	3.0	461.	1383.	---	---
	TRANS GCR SEC	7526.9	12.8	96018.	1699955.	59520.	534612.
0	FINAL TRANSFORMED COMPOSITE GIRDER PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTIONS)				
	AREA	YB	[$I_{TOT} = I_{G1} + A_{G2} - S_{GM} (A_Y * Y_B)$]				
	A _Z	I _{G1}	I _{TOT}				
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)	(IN ⁴)	
	TRANS COMP GCR	7526.9	12.8	96018.	1699955.	59520.	534612.
	G^*Y^2/R^{**2}	=	1.8192	$(E^*X^2)/R^{**2}$	=	1.7523	

STEELING W/O PRESTRESS - BEING FINAL TRANSFORMED CROSS-SECTIONS

	MOMENT (K')	FT (PSI)	FB (PSI)
DL GIRDERS	3457.	1028.	-890.
DL SLAB + FINISH	0.	0.	0.
ADDED DL	450.	134.	-129.
TOTAL DL	3907.	1162.	-1119.
CLAYTON	2237.	665.	-641.
TOTAL DL + (DL+2)	---	1826.	-1759.

DESIGN STRESS - F DES (PSI) (GRADE OF THE FLOORING)

$$F_D = 5 * \text{SQRT}(F'_C) * \text{ALL. WEL. FACTOR} = 1870. \quad F_D (\text{DL} + \text{ADD. DL}) = 1119.$$

PRESTRESSED GIRDER ANALYSIS

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PRESTRESSING FORCE - PREDETERMINED

$$\text{ALLOW. TENSION} = (417 * .75) - 35.00 = 10100. (\text{PSI}) \quad \text{FINAL PRESTRESS FORCE} - P_F = 429023. (\text{LBS})$$

$$\text{AREA OF PRESTRESSING STEEL} - A_{PS} = 25.62 (\text{SQ}) \quad \text{INITIAL PRESTRESS FORCE} - P_I = 485430. (\text{LBS})$$

STEELING WITH PRESTRESS - BEING FINAL TRANSFORMED CROSS-SECTIONS (PSI)

AT MIDSPAN	TOP FIBER	BOTTOM FIBER
Ø STEELING	(FTT) 499.	(FTB) 785.
Ø DL GIRDERS-SLAB	(FTT) 541.	(FTB) 573.
Ø FULL DES. LOAD	(FTT) 1359.	(FTB) -190.

CONCRETE STRESS REQUIRED (PSI)

$$F_{CI} = F_{MAX}/.60 = 1490. \quad F_C = F_{MAX}/.60 = 3398. \quad \text{FORMS (PSI)} = 4000.$$

DEFLECTIONS (INCHES) (DOWNWARD POSITIVE)

$$D_{11} = -1.1925 \quad D_{12} = 1.1624 \quad D_{13} = -.4300 \quad D_{14} = -.3823 \quad D_{15} = .0000 \quad D_{16} = .1513 \quad D_{17} = -.2516$$

CHECK ULTIMATE BENDING MOMENT

$$\text{REQ'D. MOM.} = .177 \quad \text{ULT. RESIST. MOMENT (K-FT)} = 10066.29$$

$$\text{FUD (PSI)} = 24370. \quad \text{N.A. Ø ULTIMATE LOAD (DS)} = 3.62$$

PRESTRESSED GIRDER ANALYSIS

PAGE 5

RESULTS - FINAL PRESTRESSING FORCE CALCULATED

GIRDER NO.	STRESS		CONCRETE STRESS		FINAL FORCE		ULTIMATE MOMENT		DEFLECTIONS (USE MM)				SECTION PROPERTIES					
	INITIAL (PSI)	FINAL (PSI)	TOP (PSI)	BOTTOM (PSI)	TOP (PSI)	BOTTOM (PSI)	APPLD (K-FT)	REQD (K-FT)	D11	D12	D13	D14	I (IN4)	A (IN2)	YS (IN)	Z (IN4)	YS (IN)	
0 1	499.	785.	1359.	-190.	1.49	3.40	4292.	5914.8	10056.3	-1.13	-0.36	.000	-0.19	534621	7527	12.8	534621	12.8

NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD

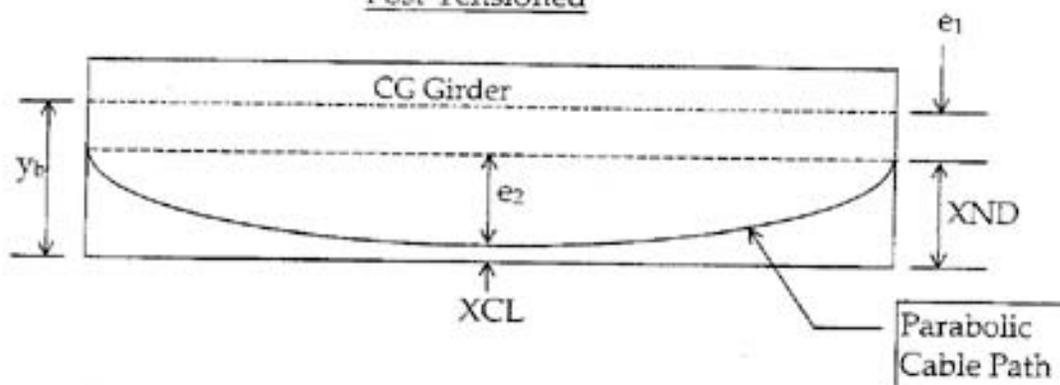
NOTE: DEFLECTIONS ARE BASED ON INPUT P-S F-C

NOTE: FOR P-S CONCRETE F'C USE 4000. PSI

Appendix 1 - Explanation of Deflections

1. Single Girder with Composite Slab

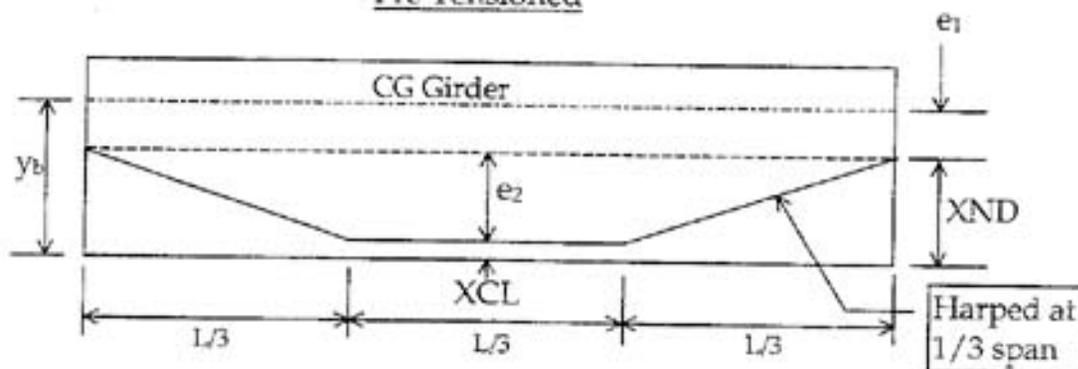
Post-Tensioned



- a) Deflection due to prestress only:

$$DPI = \frac{P_i L^2}{48 EI} (5e_2 + 6e_1)$$

Pre-Tensioned



- b) Deflection due to prestress only:

$$DPI = \frac{P_i L^2}{216 EI} (23e_2 + 27e_1)$$

$$e_1 = y_b \cdot XND \quad \text{and} \quad e_2 = XND - XCL$$

- c) Deflection due to dead load of girder:

$$DG = \frac{5W_G L^4}{384 EI} = \frac{5M_1 L^2}{48 EI}$$

Simply supported

$$M_1 = \frac{W_G L^2}{8}$$

due to the weight of the girder

d) Initial deflection - deflection due to P/S + deflection due to DI:

$$DI = DPI + DG$$

e) Deflection due to long term losses + creep:

$$DPF = - (0.89 \times DPI - DG) \times 1.5$$

f) Deflection due to dead load of slab:

$$DSLAB = \frac{5 W_s L^4}{384 EI} = \frac{5 M_2 L^2}{48 EI}$$

$$M_2 = \text{Moment due to weight of slab}$$

g) Final Deflection:

$$DF = DPI + DPF + DSLAB$$

2. Box Girder

$$\text{DEF 1 } DI = (5e_2 + 6e_1) \frac{P_1 L^2}{48 EI} \qquad DPI = (5e_2 + 6e_1) \frac{P_1 L^2}{48 EI}$$

$$\text{DEF 2 } DSLAB = DG$$

$$\text{DEF 3 } DF = \frac{5 WL^4}{384 EI} = \frac{40 M_3 L^2}{384 EI}$$

$$\text{Here } M_3 = \text{Moment due to added DL}$$

Appendix 2

I - How to change the file created by a version older than 2.00 version into the file usable by 2.00 or later versions

By adding "Units" card, a file created by a version older than 2.00 version can be used by 2.00 or later versions. For illustration, one card from the text file of the old version of example problem 5 after adding the Units card is shown here. This Units card should have all variables as shown here. After adding the Units card, variables in the Units card can be changed with the help of the panel program. In this way, a file in the fixed format can be changed into a file in the free format.

Units Card to be added in the file created by a version older than 2.00 version

'Units' 'E' 'E' 'E'
 1 0 00+ 1 800 67 0 0 0 0 00 7691

II - Panel Program Operation

Following keys are used for the following functions when the panel program is used:

<u>Key(s)</u>	<u>Function</u>
F3	A question will appear on the screen whether user decides to quit the panel program. If F3 pressed again, the panel program will be quitted. Otherwise, the panel program will return to the previous status.
F7	Displays error messages in the current panel's data for certain errors. In absence of such errors, the panel before the current panel is displayed or, if the current panel is the first panel, the current panel is displayed.
F8	Displays error messages in the current panel's data for certain errors. In absence of such errors, the panel after the current panel is displayed or, if the current panel is the last panel, the current panel is displayed.

Key(s)	Function
F9	A question will appear on the screen whether user decides to save date used in the panel program. If F9 pressed again, the date in the panel program will be saved and the panel program will be quitted. Otherwise, the panel program will return to the previous status.
→	Shifts the cursor one space right in the current panel. If the cursor is on the last space of the current panel, the cursor shifts to the top of the current panel.
←	Shifts the cursor one space left in the current panel. If the cursor is on the first space of the current panel, the cursor shifts to the bottom of the current panel.
↑	Shifts the cursor one field left in the current panel. If the cursor is on the first field of the current panel, the cursor shifts to the last field of the current panel.
↓	Shifts the cursor one field right in the current panel. If the cursor is on the last field of the current panel, the cursor shifts to the first field of the current panel.
Enter (On numeric pad)	Checks the certain errors in the current panel's data, moves the cursor to the next blank line and displays the current panel.
Enter (Below Backspace)	Moves the cursor to the next line and displays the current panel.
Backspace	Erases the character/number/letter at the cursor and shifts the cursor one position back. If the cursor is at the first position of the field, the cursor shifts to the first position of the previous field.
Tab	It has the same effect as ↓ has.

Warning: Do not use any other key which is not mentioned here. Otherwise, any other key may cause interruption or termination of the panel program. For example, **Delete**, **End**, **Esc**, **F10**, **F11**, **F12**, **Home**, **Insert**, **Page Up**, **Page Down**, or **Pause** may cause interruption or termination of the panel program. If the panel program is interrupted or terminated, you may start the panel program again but the current data in the panels may be lost.

III- Tips for Free Formatted Version

1. Any data entered after a completely blank line is not saved.
2. All comments in the input file to the panel program are ignored. The text file, or the output file of the panel program shall have the comments decided by the panel program.
3. The data in the panels which are viewed might have different significant digits. Most of remaining data do not change their significant digits. Thus, it is recommended to see the panels which are necessary to view. In this way, significant digits of remaining data are not lost.
4. The panel program checks the certain errors in a line if all previous lines in the panel have the data. If the first field of the line is a trial number or section number, the first field and any other field should have the data in order to have this check. Otherwise, any field should have the data in order to have this check.

Appendix 3

Included in this section are full-sized forms of panels in English and Metric units to be photocopied and used by the engineers.

- A Prestressed Girder Analysis
- B Superstructure Section
- C Sections by Parts
- D Section Dimensions - Symmetric Sections
- E Section Dimensions - Unsymmetric Sections

Trial No.	Section No.	Girder Properties						Prestress Data						Moments																																																																															
		Structure (H2)		Girder (H3)		Girder Spacing (S)**		PS f'c		Composite Slab		Tension		XCL	XND	Low Lax	Allowable Tension Factor	Final Prestress Force (kips)	Live Load Lanes	Note A	Note B	Note C	Note D																																																																						
		Depth (in)	(H2)	Depth (in)	(H3)	(S)	(in)	Thick (in)	(H1)	Width (in)	Super (in)	f'c (ksi)	f'c (ksi)											f's (ksi)	(in)	(in)	(in)	(kip-ft)	(kip-ft)	(kip-ft)	(kip-ft)	(LL+1)M Applied to Final Section																																																													
3	4	8	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100

SC 1791

INSTRUCTIONS:

If an Office of Structures standard section is used, enter correct code from table at right in Type and omit any supplementary section sheets. If other than a standard section is used, it must be referred to by "Section No." if described by "By Parts" of "Superstructure Sections". It must be described on the "Section Dimensions" sheet.
 Prestress: If the system is pretensioned, enter a 1. Leave column blank for post-tension.
 XCL is the dimension from bottom of girder to CGS at centerline of span, XND at end. If deflections are not desired, enter 99 in XND.
 For low relaxation, enter a 1.
 Allowable tension factor in length; 10 to 100%; .05 to 50%.

** Girder spacing (S) is width of the structure subjected to the HS20-44 live load.

- NOTES:** (See Memo to Designers 20-16)
- A Omit when simple span DLM is due to the described section.
 - B Omit when simple span DLM is due to the described final section. DLM should not include DLM of note A.
 - C Show the additional DLM applied to the final girder section.
 - D Omit when max HS20-44 simple span moment is that for the described girder spacing (S) and span length (L). The distribution factor assumed is S/5.5.
 Omit Live Load Lanes to use S-over Distribution Factor.

Code	Standard Type
1	"I" Girder

